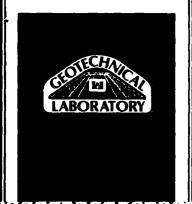


MICROCOPY RESOLUTION TEST CHART
NATIONAL BUREAU OF STANDARDS-1963-A









DTIC FILE COPY





REVIEW OF RIGID PAVEMENT DESIGN FOR VEHICULAR PARKING AREAS

by

John C. Potter

Geotechnical Laboratory

DEPARTMENT OF THE ARMY
Waterways Experiment Station, Corps of Engineers
PO Box 631
Vicksburg, Mississippi 39180-0631



February 1985 Final Report

Approved for Public Release; Distribution Unlimited



Prepared for

DEPARTMENT OF THE ARMY US Army Corps of Engineers Washington, DC 20314-1000

85 05 02 042

Destroy this report when no longer needed. Do not return it to the originator.

The findings in this report are not to be construed as an official Department of the Army position unless so designated by other authorized documents.

The contents of this report are not to be used for advertising, publication, or promotional purposes. Citation of trade names does not constitute an official endorsement or approval of the use of such commercial products.

Unclassified
SECURITY CLASSIFICATION OF THIS PAGE (When Date Entered)

REPORT DOCUMENTATION PAGE	READ INSTRUCTIONS BEFORE COMPLETING FORM			
1. REPORT NUMBER 2. GOVT ACCESSION NO. Miscellaneous Paper GL-85-2	FECIPIENT'S CATALOG NUMBER			
4. TITLE (and Subtitio)	5. TYPE OF REPORT & PERIOD COVERED			
REVIEW OF RIGID PAVEMENT DESIGN FOR VEHICULAR PARKING AREAS	Final report			
PARRING AREAS	6. PERFORMING ORG. REPORT NUMBER			
7. AUTHOR(e)	B. CONTRACT OR GRANT NUMBER(*)			
John C. Potter				
9. PERFORMING ORGANIZATION NAME AND ADDRESS US Army Engineer Waterways Experiment Station	10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS			
Geotechnical Laboratory				
PO Box 631, Vicksburg, Mississippi 39180-0631				
11. CONTROLLING OFFICE NAME AND ADDRESS	12. REPORT DATE			
DEPARTMENT OF THE ARMY US Army Corps of Engineers	February 1985			
Washington, DC 20314-1000	13. NUMBER OF PAGES 10			
14. MONITORING AGENCY NAME & ADDRESS(If different from Controlling Office)	15. SECURITY CLASS. (of this report)			
	**-1			
	Unclassified			
	15a. DECLASSIFICATION DOWNGRADING SCHEDULE			
16. DISTRIBUTION STATEMENT (of this Report)	<u> </u>			
Approved for public release; distribution unlimited	1.			
inproved for public followed, diberious difficulties	••			
17. DISTRIBUTION STATEMENT (of the abstract entered in Block 20, If different fro	m Report)			
18. SUPPLEMENTARY NOTES				
Available from National Technical Information Serv	ice. 5285 Port Royal Road.			
Springfield, Virginia 22161.	,,,,			
19. KEY WORDS (Continue on reverse side if necessary and identify by block number				
Parking areas				
Pavement design				
Rigid pavement				
20. ABSTRACT (Continue on reverse side if recovery and identity by block number) ➤ This paper documents proposed changes to TM.				
for Roads, Streets, Walks, and Open Storage Areas.				
in the areas of impact effects, the coverage versus				
and effects of high-strength subgrades. Additionally, the criteria for design				
thickness of vehicular parking areas should be chan	nged to account for a differ-			
ent slab loading condition, including load transfer at joints. These changes establish a rational and consistent basis for CE design of rigid airfield and				
nonairfield pavements.	or retra griffeld and			
DD FORM 1473 EDITION OF I MOVES IS DESCRIPTE				

Unclassified
SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered)

PREFACE

This review was conducted by the Geotechnical Laboratory (GL), US Army Engineer Waterways Experiment Station (WES), during the period February to September 1984. It was sponsored by the Office, Chief of Engineers, US Army, under the work effort "Review of Rigid Pavement Design of Vehicular Parking Areas" of the Facilities Investigation and Studies (FIS) Program.

The review was conducted under the general supervision of Dr. W. F. Marcuson III, Chief, GL; Dr. T. D. White, Chief, Pavement Systems Division (PSD); Mr. Hugh L. Green, Chief, Engineering Analysis Group; and Mr. D. M. Ladd, Chief, Criteria Development Unit. The review was conducted by Dr. John C. Potter, PSD, who is the author of this paper.

The Commanders and Directors of WES during this review were COL Tilford C. Creel, CE, and COL Robert C. Lee, CE. Technical Director was Mr. F. R. Brown.

Acces	sion For	
NTIS	GRA&I	R
DTIC :	TAB	
Unaren	ounced	
Just	"ication	
	ibution/	
	Avail and.	
Dist	Spec. AJ	
A-1		



CONTENTS

						Page
PREFACE	•	•	•	•		1
CONVERSION FACTORS, US CUSTOMARY TO METRIC (SI) UNITS OF						
MEASUREMENT	•	•	•	•	• •	3
PART I: INTRODUCTION	•		•	•		4
PART II: RECOMMENDED CHANGES	•	•	•	•		5
Roads, Streets, and Vehicular Parking Areas						
Vehicular Parking Areas Only	•	•	•	•		7
PART III: SUMMARY	•	•	•	•		9
REFERENCES						10

CONVERSION FACTORS, US CUSTOMARY TO METRIC (SI) UNITS OF MEASUREMENT

US customary units of measurement used in this report can be converted to metric (SI) units as follows:

Multiply	Ву	To Obtain
inches	2.54	centimetres
miles (international) per hour	1.609344	kilometres per hour
pounds (force) per cubic inch	0.271477	newtons per cubic metre
pounds (force) per square inch	6.894757	kilopascals

REVIEW OF RIGID PAVEMENT DESIGN FOR VEHICULAR PARKING AREAS

PART I: INTRODUCTION

- 1. This paper describes the proposed changes to the design criteria used in TM 5-822-6, "Rigid Pavements for Roads, Streets, Walks, and Open Storage Areas" (Office, Chief of Engineers, Department of the Army 1984). The March 1984 edition of the TM recommends that rigid pavements for roads, streets, and vehicular parking areas be designed using criteria developed for roadways in 1961 (US Army Engineer Division, Ohio River, 1961). Since then, experience with test sections and in-service pavements has added to knowledge of pavement mechanics. The old criteria appear conservative, especially for vehicular parking areas, which are loaded differently from roadways.
- 2. The purpose of this review was to investigate the potential for reducing pavement design thicknesses, particularly for vehicular parking areas based on information developed since 1961. Topics given particular attention were (a) impact, (b) coverage versus thickness relationship, (c) effects of high-strength subgrades, (d) slab loading conditions, and (e) traffic channelization. The first three of these topics apply to roads and streets as well as vehicular parking areas. The latter two topics apply only to design of vehicular parking areas.
- 3. These changes will reflect current trends being pursued in rigid pavement design and will make the US Army Corps of Engineers (USACE) design philosophy for rigid airfield and nonairfield pavements consistent.

PART II: RECOMMENDED CHANGES

Roads, Streets, and Vehicular Parking Areas

- 4. The design criteria for roads, streets, and vehicular parking areas should be modified in the areas of impact, coverage versus thickness relationship, and effects of high-strength subgrades.
- 5. Tests have shown that test vehicles on pavements experience impact effects. However, the pavements themselves do not. The axle loads of a moving truck cause smaller stresses than those of a stopped truck. In the Maryland Road Test (Highway Research Board 1952), stresses were measured at pavement edges and tranverse joints for speeds up to 40 mph.* Stresses at outside edges decreased 30 percent when truck speeds were raised from creep to 40 mph. Stresses at transverse joint edges decreased 15 percent at 40 mph. Stresses were decreased still more when 3/4-in. boards were placed on the pavement to simulate joint faulting. Similar results were reported from the American Association of State Highway Officials (AASHO) Road Test (Highway Research Board 1962). This agrees with USACE experience and with the current philosophy for design of airfield pavements. Therefore, use of an impact factor is not justified.
- 6. Previously, the standard thickness (for 5,000 coverages) was calculated using a combined design factor of 1.55. This included a 25 percent increase in the static load for impact and a 30 percent increase for load repetition. Eliminating the impact factor reduces the combined design factor to 1.3, for a thickness reduction of about 11 percent.
- 7. The percent standard thickness versus coverage relationship should be eliminated and a design factor versus coverages relationship established. This will allow the actual, rather than standard, design thickness to be calculated from the thickness equation, by replacing the old standard thickness design factor of 1.3 with the design factor determined from the new design factor versus coverage relationship. Using the new design factor versus coverages relationship for airfield pavements (revised under the USACE Facilities Investigation and Studies Program work effort "Review of Rigid Pavement Design Criteria") will incorporate data not included in development of the

^{*} A table of factors for converting US customary units of measurement to metric (SI) units is presented on page 3.

percent thickness versus coverages relationship, and will preserve the consistency between the airfield and nonairfield rigid pavement design criteria.

- 8. The change in thickness of roads, streets, and vehicular parking areas resulting from this modification depends upon the design traffic coverage level. For low coverage levels, the design thickness is reduced as much as 4 percent. For high coverage levels, the thickness is increased as much as 19 percent.
- 9. Current airfield pavement design includes a thickness reduction for high-strength subgrades. This reduction is based on USACE experience, and its validity is illustrated by the performance of concrete block pavements on high-strength subgrades. This same reduction (Hutchinson 1966) has been applied for roads, streets, and vehicular parking areas. The amount of thickness reduction depends upon the value of the modulus of soil reaction, k. For k values above 100 pci, the reduction in design thickness varies from zero (at k = 200 pci) to a maximum of 19.1 percent (at k = 500 pci).
- 10. The cumulative change in design thicknesses of roads and streets depends upon the coverage level (design index) and subgrade strength. It varies from an increase of 9 percent to a decrease of 19 percent for typical values of design parameters. The new rigid pavement design curves for roads and streets are shown in Figure 1.

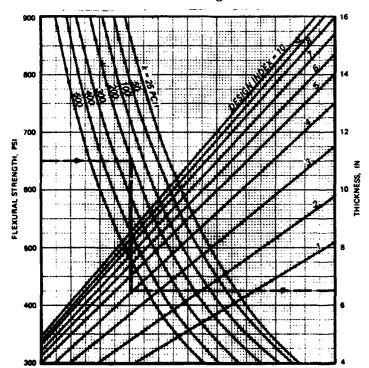


Figure 1. Rigid pavement design curves for roads and streets

Vehicular Parking Areas Only

- 11. A survey of parking areas and traffic patterns on various military installations was conducted by the project engineer. One observation from this survey is that traffic avoids the free edges of parking lot pavements. This phenomenon is caused by curbs, fences, buildings, soft ground, etc., common along the edges of parking areas. Since the free edges are not heavily trafficked, the maximum damage to parking lot pavements results from stresses caused by traffic loads along interior joints rather than along free edges. These interior joints are load transfer joints. A 25 percent load transfer is used for interior joints of airfield pavements (Parker et al. 1979). Therefore, the maximum design stresses in parking lot pavements can be estimated at 75 percent of the free edge stresses. Using these reduced stresses allows a design thickness reduction of about 17 percent.
- 12. No changes have been made in the traffic distribution assumptions used for the design of roads and streets. Several observations from the parking lot survey support this decision. First, parking lot stall and aisle widths are similar to traffic lane widths on roads and streets. Second, traffic flow in parking areas is restricted to road and street dimensions at entrances and exits. Thus, parking lot traffic distributions are similar to those of roadways in the highly trafficked areas. Finally, parking area flow patterns are subject to change over the life of the facility. Hence, distribution assumptions dependent upon permanent traffic flow patterns would be inappropriate.
- 13. The cumulative decrease in parking area design thickness, depending upon coverage level and subgrade strength, is 12 to 27 percent for typical values of design parameters. The new rigid pavement design curves for vehicular parking areas are shown in Figure 2.

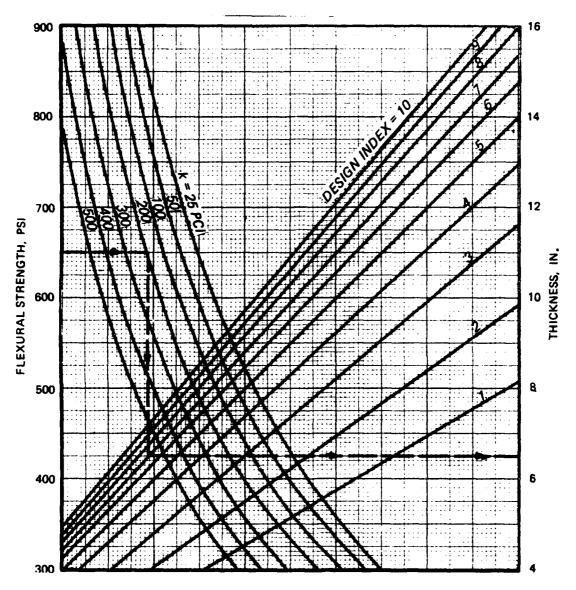


Figure 2. Rigid pavement design curves for parking areas

PART III: SUMMARY

- 14. The proposed design criteria for roads, streets, and vehicular parking areas include modifications to eliminate the impact factor, use a design factor based on new and reevaluated test section data, and provide for thickness reductions for high-strength subgrades. In addition, vehicular parking areas are designed using the stresses computed for interior, load-transfer joints, rather than for free edges.
- 15. The thickness changes produced by these modifications are shown in Table 1. Note that the actual, cumulative changes are limited by the minimum allowable thickness for plain concrete pavement of 6 in., specified by TM 5-822-6 (Office, Chief of Engineers, Department of the Army 1984).

Table 1
Percent Change in Thickness

Source	Roads and Streets	Vehicular Park- ing Areas	Remarks
Impact factor	-11	-11	
Percent standard thickness versus coverages	-4 to +19	-4 to +19	Depends upon coverage level
High-strength subgrade	-19.2 to 0	-19.2 to 0	Depends upon subgrade strength
Load transfer	0	-17	
Theoretical total	-31 to +6	-43 to -12	
Actual total	-19 to +9	-27 to -12	For resonable values of material properties

^{16.} These changes establish a consistent basis for USACE design of all rigid pavements and reflect the current doctrine and state of the art.

REFERENCES

Highway Research Board. 1952. "Road Test One-MD," Special Report No. 4, National Academy of Sciences--National Research Council, Washington, DC.

. 1962. "The AASHO Road Test," Special Report No. 61E, National Academy of Sciences--National Research Council, Washington, DC.

Hutchinson, R. L. 1966. "Basis for Rigid Pavement Design for Military Airfields," Miscellaneous Paper No. 5-7, US Army Engineer Division, Ohio River Laboratory, Cincinnati, Ohio.

Office, Chief of Engineers, Department of the Army. 1984 (Mar). "Rigid Pavements for Roads, Streets, Walks, and Open Storage Areas," Technical Memorandum 5-822-6, Washington, DC.

Parker, F., Barker, W. R., Gunkel, R. C., and Odom, E. C. 1979 (Apr). "Development of a Structural Design Procedure for Rigid Airport Pavements," Technical Report GL-79-4, US Army Engineer Waterways Experiment Station, Vicksburg, Miss.

US Army Engineer Division, Ohio River. 1961 (July). "Development of Rigid Pavement Thickness Requirements for Military Roads and Streets," Technical Report No. 4-18, Cincinnati, Ohio.

END

FILMED

6-85

DTIC